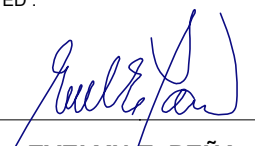


REPUBLIC OF THE PHILIPPINES
DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS
REGION IV - B
MINDORO ORIENTAL DISTRICT ENGINEERING OFFICE
MASIPIT, CALAPAN CITY, ORIENTAL MINDORO

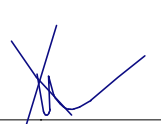
C.Y. 2025 PROJECT
DETAILED ENGINEERING DESIGN PLAN FOR
**REHABILITATION OF MAINTENANCE
SECTION AND COA BUILDING**
CALAPAN CITY, ORIENTAL MINDORO

SUBMITTED :



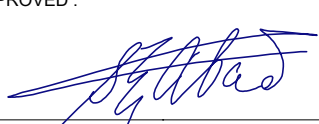
EVELYN E. PEÑA
CHIEF, PLANNING AND DESIGN SECTION

RECOMMENDED :



ALBIÉN M. MAHIA
ASSISTANT DISTRICT ENGINEER

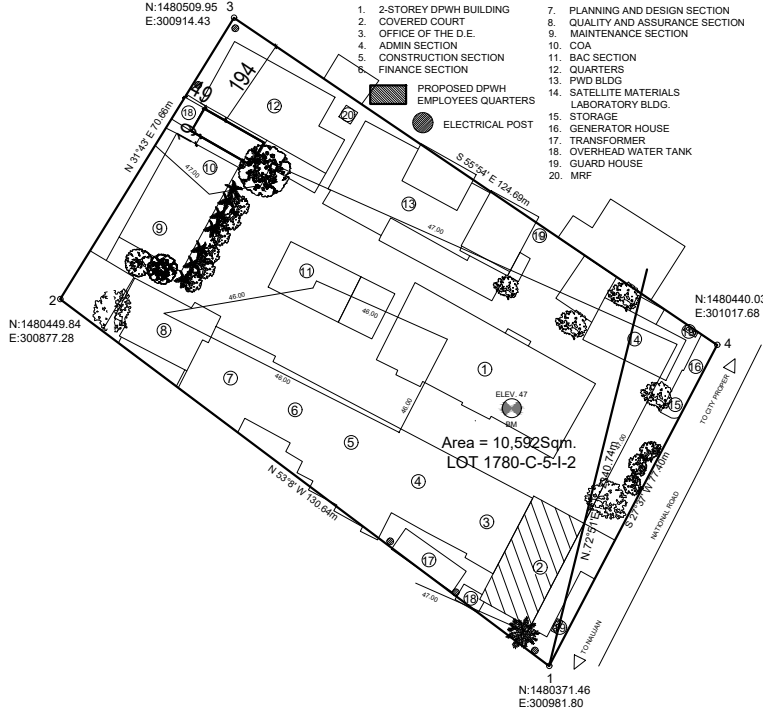
APPROVED :



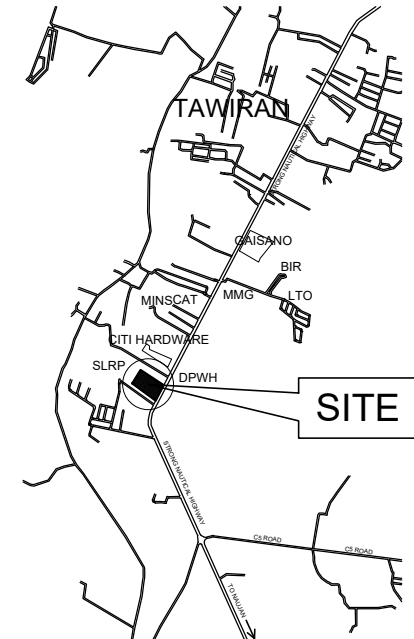
SALVADOR E. ABAD
DISTRICT ENGINEER



PERSPECTIVE
SCALE NTS.



SITE DEVELOPMENT PLAN
SCALE NTS.



VICINITY MAP
SCALE NTS.

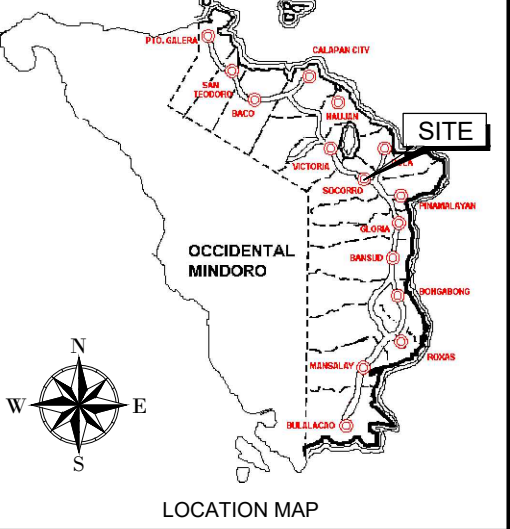
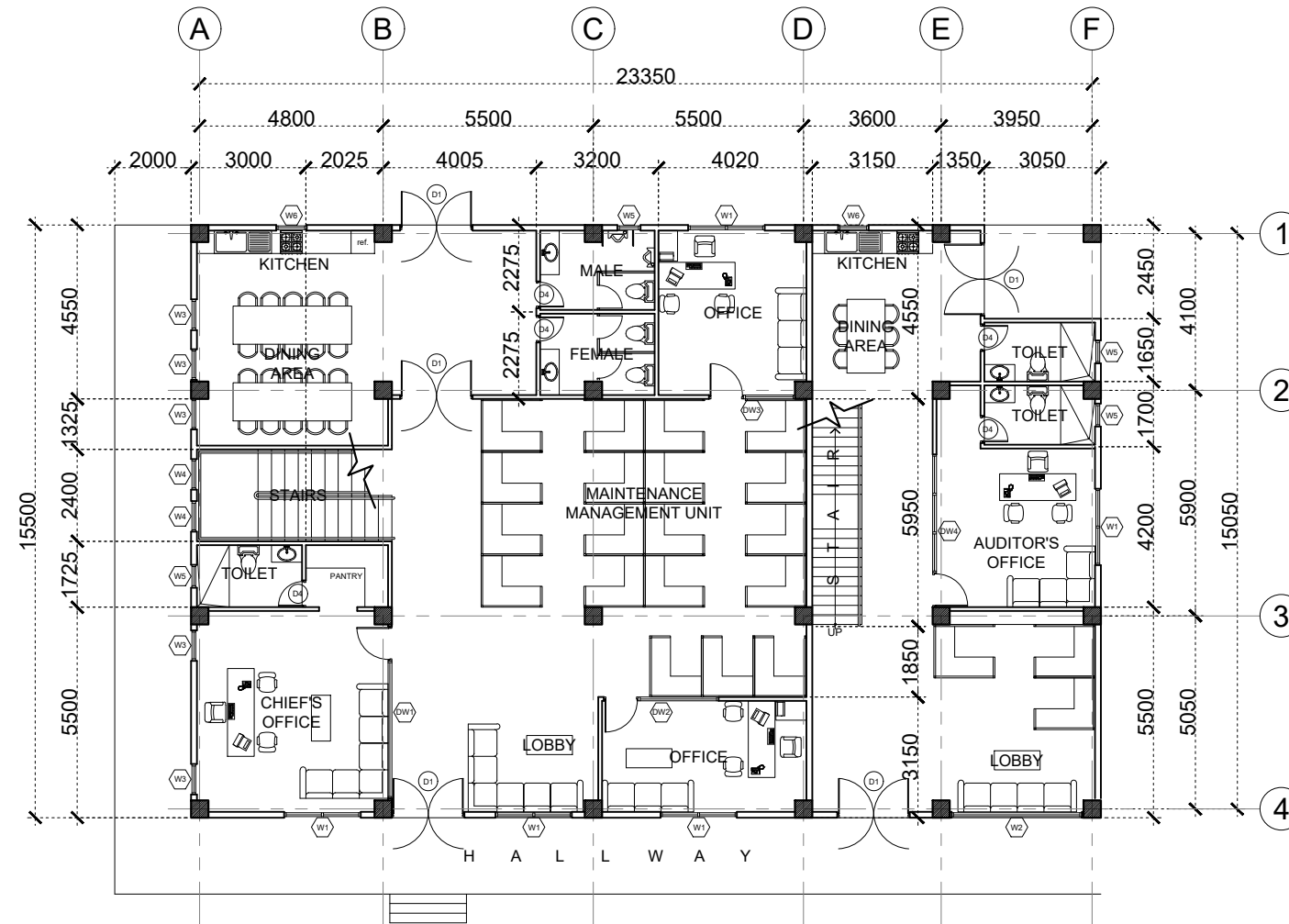


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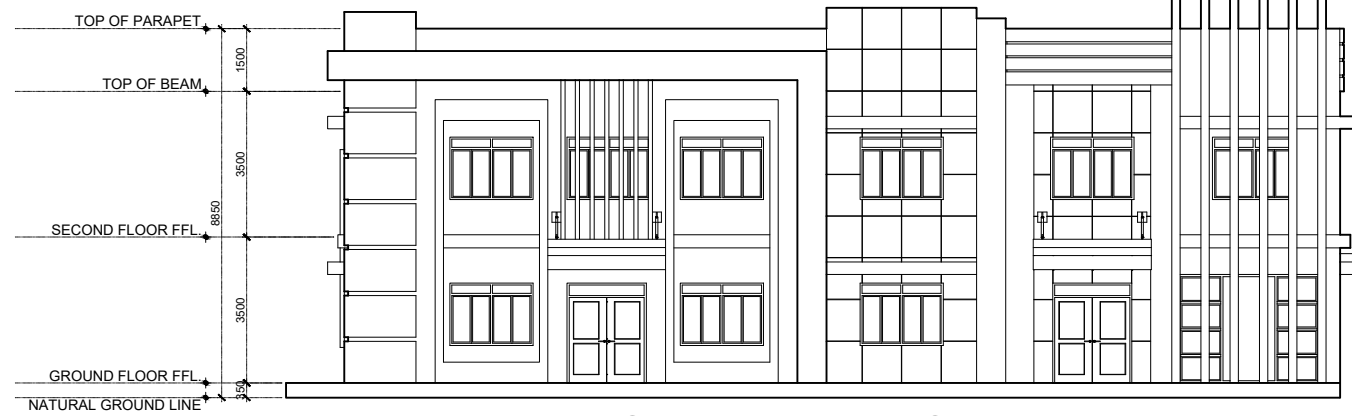


GROUND FLOOR PLAN

SCALE 1:100 MTS.

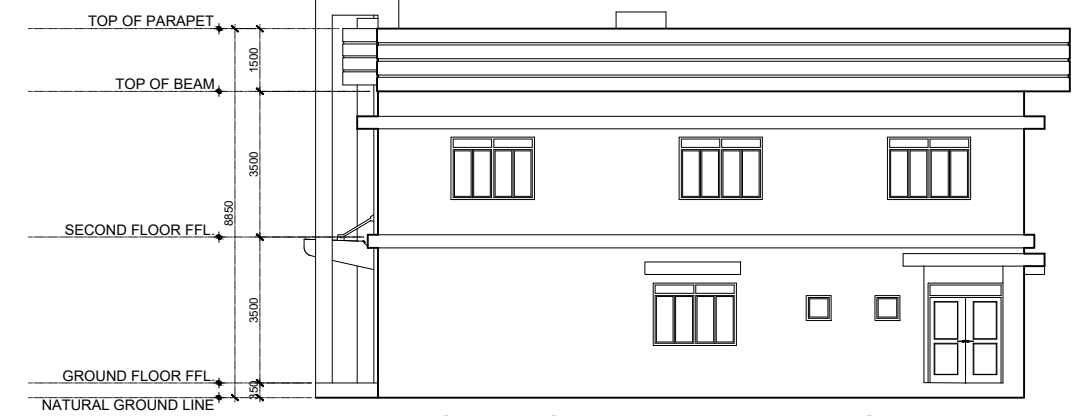
SCHEDULE OF FINISHES

- F1 600mm x 600mm GLAZED TILES
- F2 600mm x 600mm UNGLAZED TILES
- F3 300mm x 300mm UNGLAZED TILES



FRONT ELEVATION

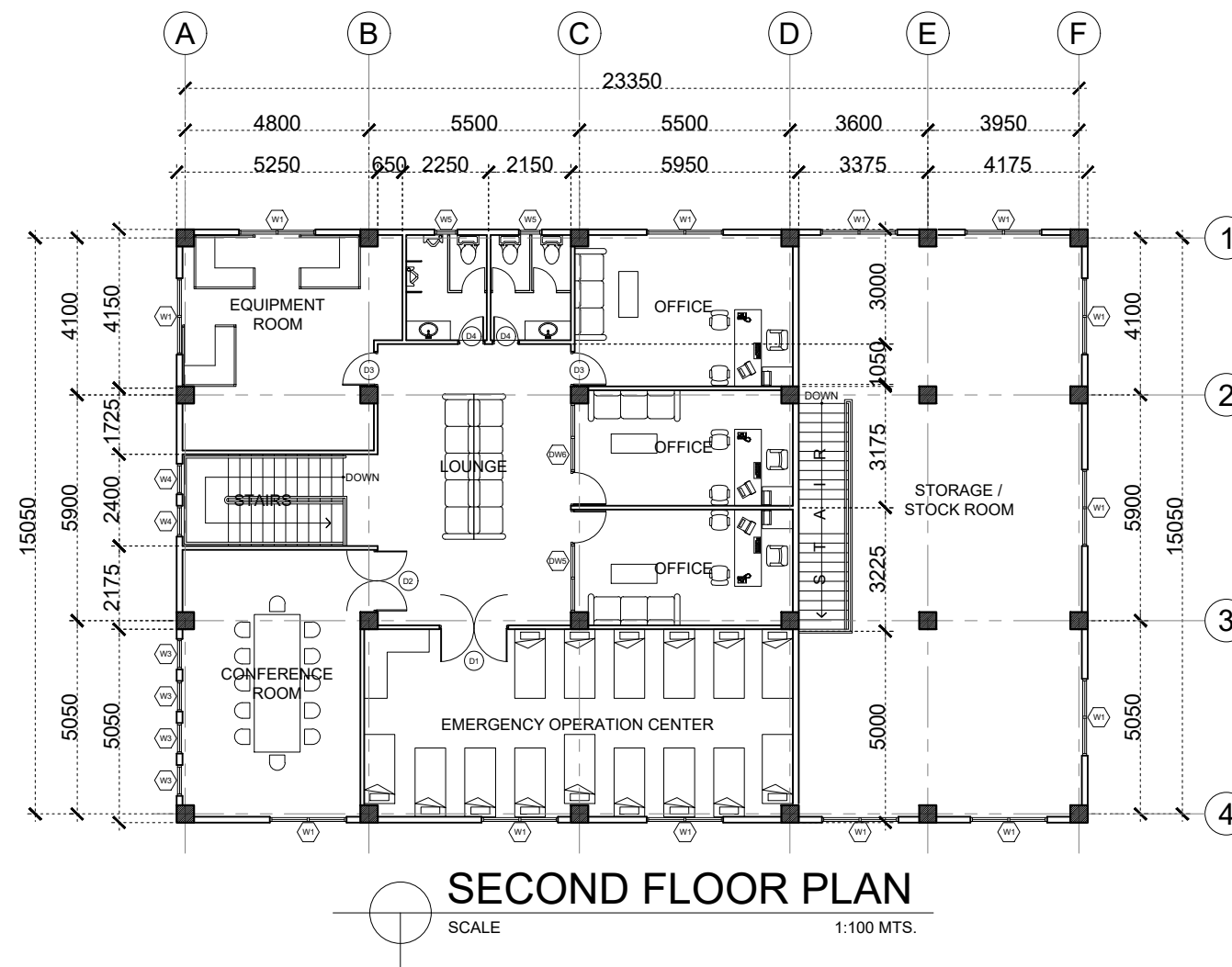
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RIGHT SIDE ELEVATION

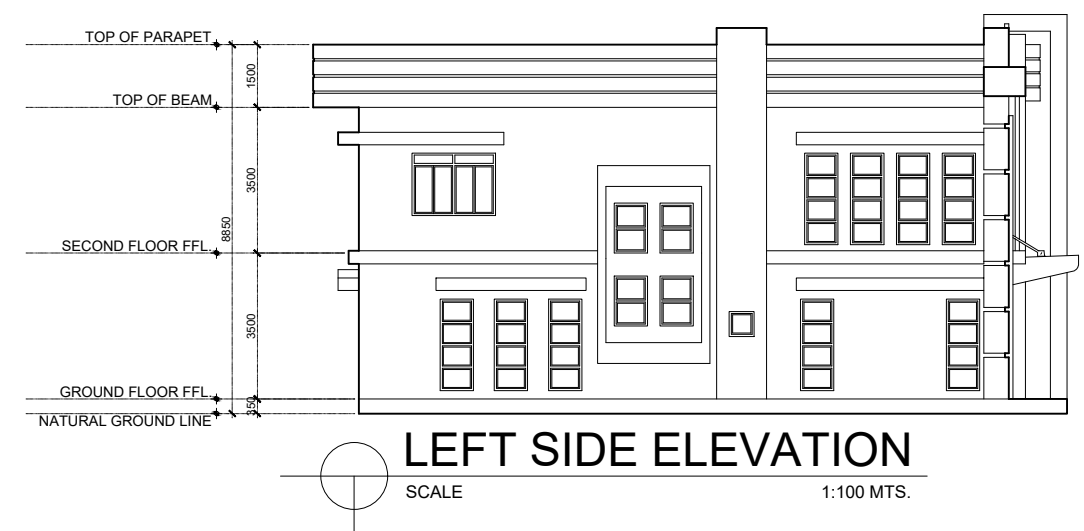
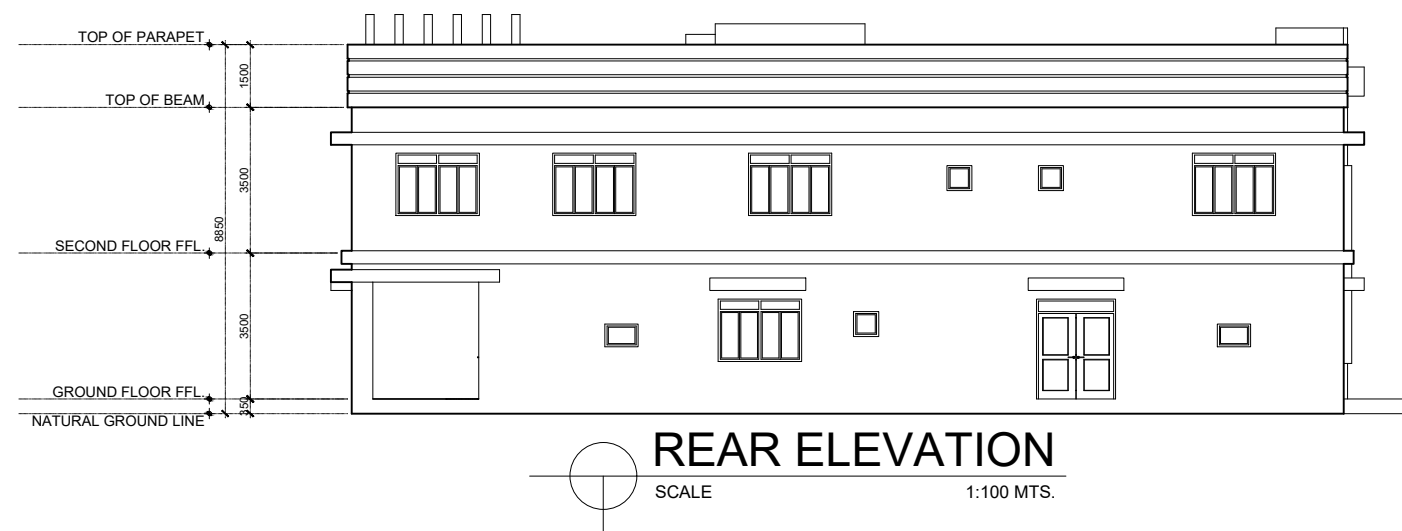
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 REPUBLIC OF THE PHILIPPINES DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS REGION IV - B MINDORO ORIENTAL DISTRICT ENGINEERING OFFICE BRGY. MASIPIT CALAPAN CITY	PROJECT :	SHEET CONTENT:	DRAFTED BY:	REVIEWED BY:	SUBMITTED BY:	RECOMMENDING APPROVAL:	APPROVED BY:	SET NO.	SHEET NO.
	REHABILITATION OF MAINTENANCE SECTION AND COA BUILDING	• GROUND FLOOR PLAN • FRONT ELEVATION • RIGHT SIDE ELEVATION	AARON LOUIE C. ALULOD ENGINEER II	DUANE ANDREW ORIEL C. ANORICO ENGINEER II	EVELYN E. PENA ENGINEER III	ALBIEN M. MAHIA ASST.DISTRICT ENGINEER	SALVADOR E. ABAD DISTRICT ENGINEER	DPWH CY-2025 MODEO	BUILDING A 2
	LOCATION: CALAPAN CITY, ORIENTAL MINDORO		PREPARED BY: ROMEO A. VILLANUEVA ARCHITECT II						

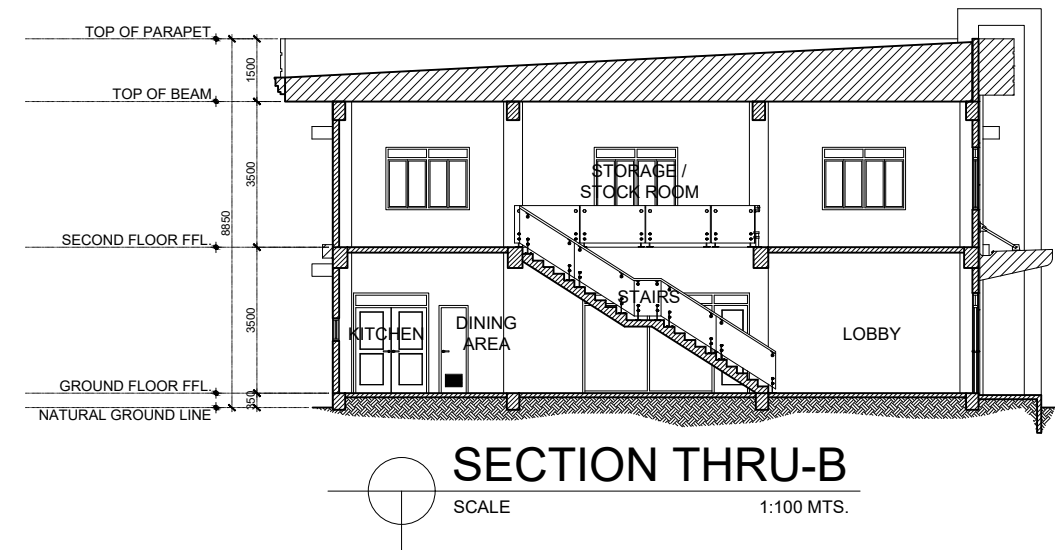
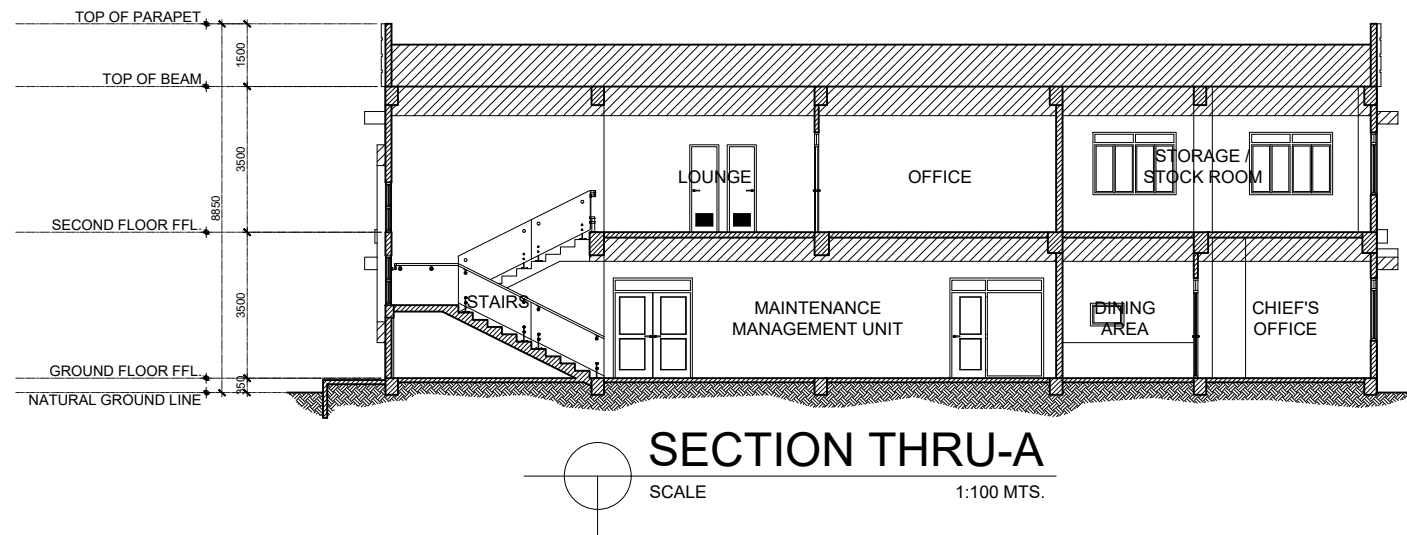
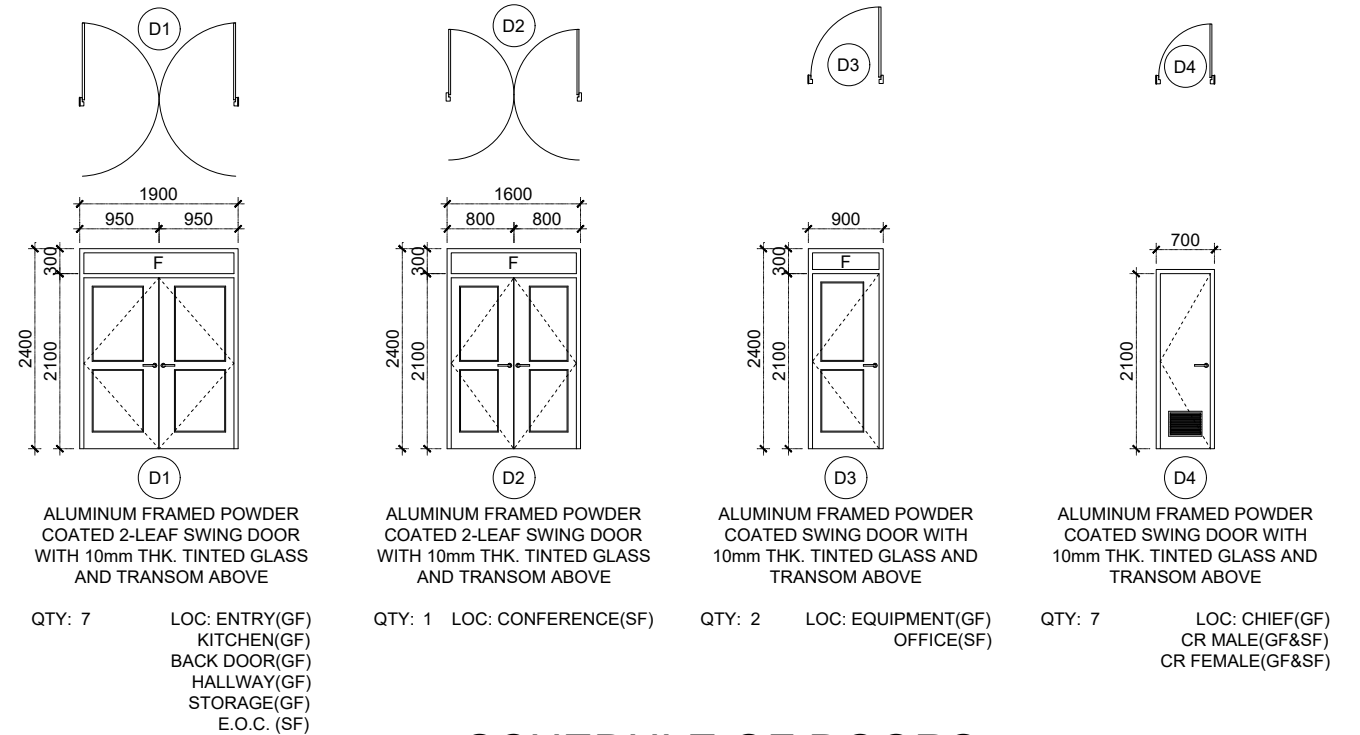
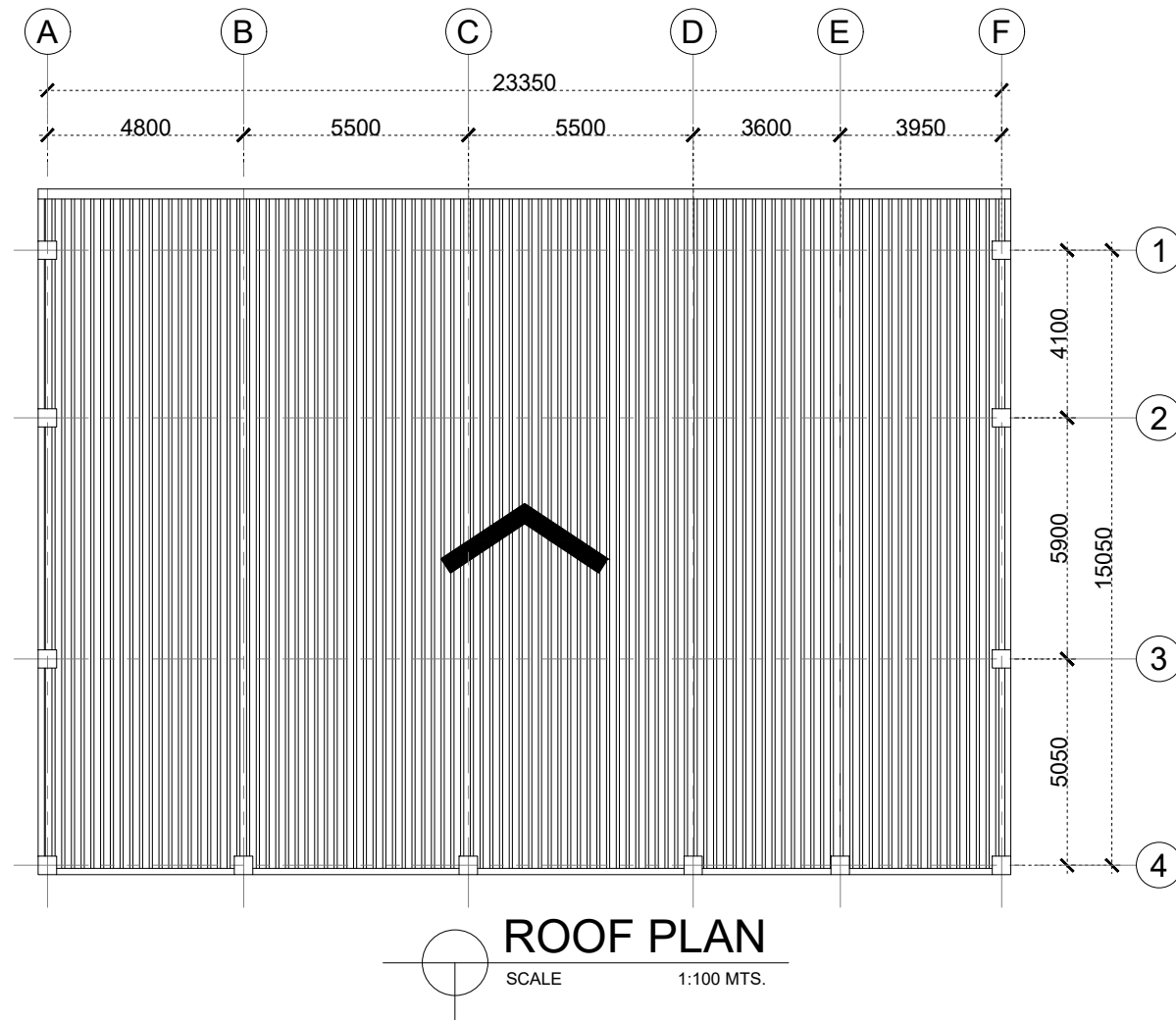


SCHEDULE OF FINISHES

- F1 600mm x 600mm GLAZED TILES
- F2 600mm x 600mm UNGLAZED TILES
- F3 300mm x 300mm UNGLAZED TILES



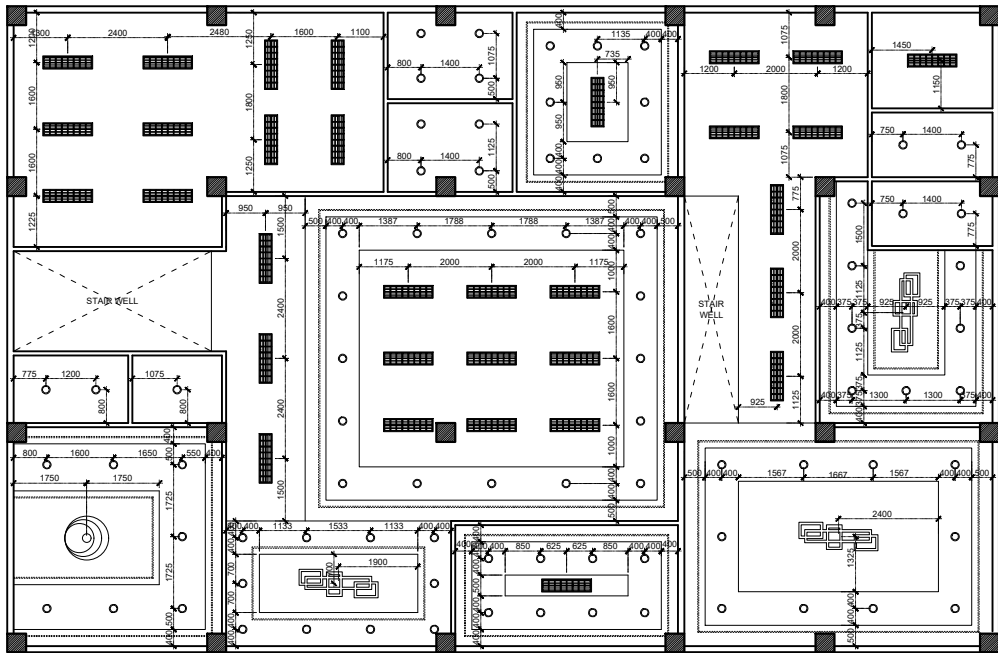
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	REHABILITATION OF MAINTENANCE SECTION AND COA BUILDING	- SECOND FLOOR PLAN - REAR ELEVATION - LEFT SIDE ELEVATION	AARON LOUIE C. ALULOD ENGINEER II	DUANE ANDREW ORIEL C. ANORICO ENGINEER II	EVELYN E. PENA ENGINEER III	ALBIEN M. MAHIA ASST. DISTRICT ENGINEER	SALVADOR E. ABAD DISTRICT ENGINEER	DPWH CY-2025 MODEO	BUILDING A 3
	LOCATION: CALAPAN CITY, ORIENTAL MINDORO		PREPARED BY: ROMEDA VILLANUEVA ARCHITECT II						



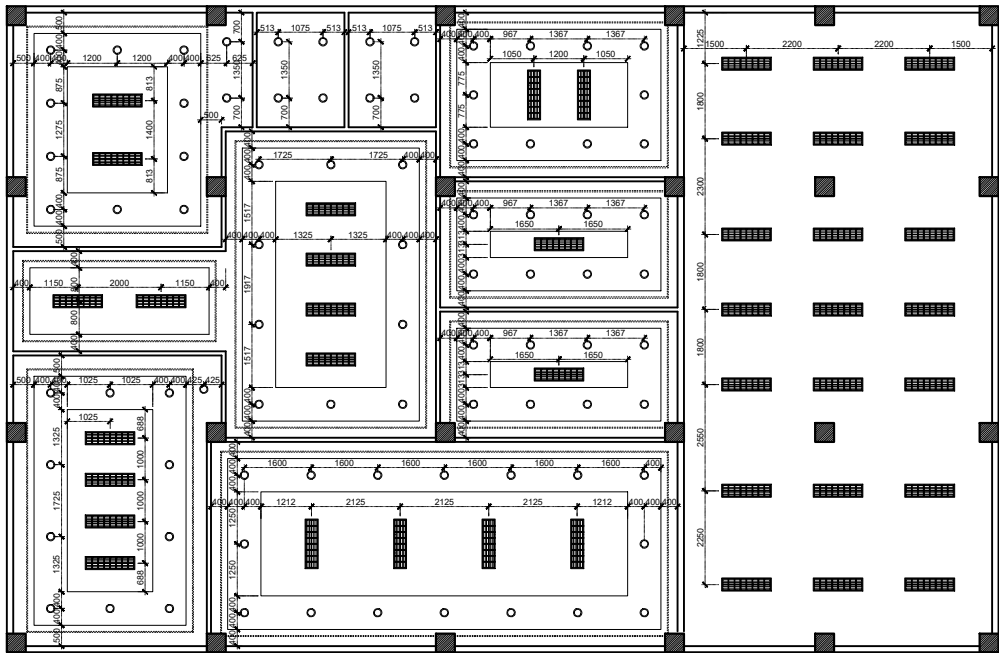
REPUBLIC OF THE PHILIPPINES DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS REGION IV - B MINDORO ORIENTAL DISTRICT ENGINEERING OFFICE BRGY. MASIPIT CALAPAN CITY	PROJECT :	SHEET CONTENT:	DRAFTED BY:	REVIEWED BY:	SUBMITTED BY:	RECOMMENDING APPROVAL:	APPROVED BY:	SET NO.	SHEET NO.
	REHABILITATION OF MAINTENANCE SECTION AND COA BUILDING	- ROOF PLAN - SECTION THRU-A - SECTION THRU-B - SCHEDULE OF DOORS	AARON LOUIE C. ALULOD ENGINEER II	DUANE ANDREW ORIEL C. ANORICO ENGINEER II	EVELYN E. PENA ENGINEER III	ALBIEN M. MAHIA ASST.DISTRICT ENGINEER	SALVADOR E. ABAD DISTRICT ENGINEER	DPWH CY-2025 MODECO	BUILDING A 4
	LOCATION: CALAPAN CITY, ORIENTAL MINDORO		PREPARED BY: ROMEO A. VILLANUEVA ARCHITECT II						

SCHEDULE OF FINISHES

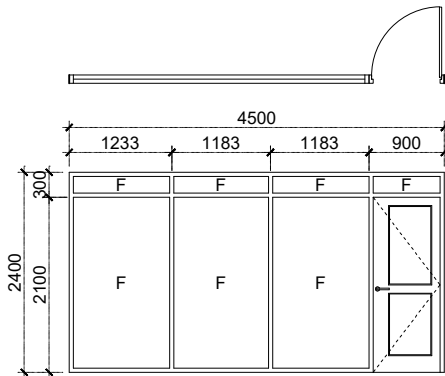
C1 45mm CEMENT FIBER BOARD



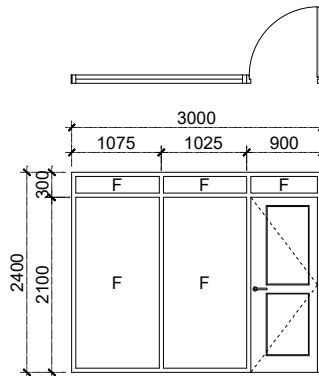
GROUND FLOOR REFLECTED CEILING PLAN
SCALE 1:100 MTS.



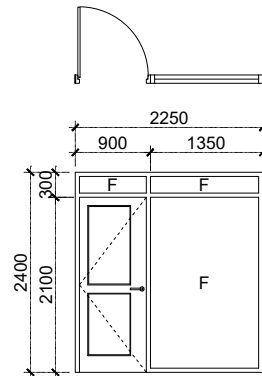
SECOND FLOOR REFLECTED CEILING PLAN
SCALE 1:100 MTS.



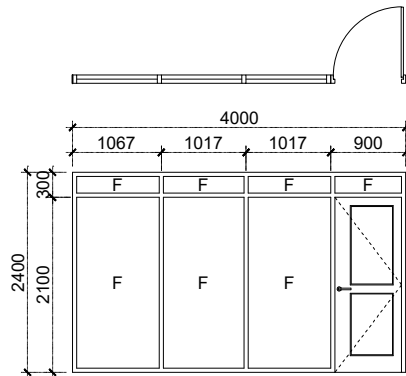
DW1
ALUMINUM FRAMED POWDER COATED SWING DOOR WITH 10mm THK. ONE-WAY GLASS AND FIXED GLASS WALL WITH TRANSOM ABOVE
QTY: 1 LOC: CHIEF'S OFFICE (GF)



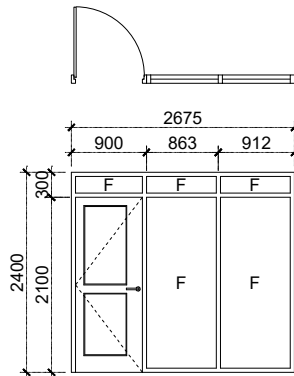
DW2
ALUMINUM FRAMED POWDER COATED SWING DOOR WITH 10mm THK. ONE-WAY GLASS AND FIXED GLASS WALL WITH TRANSOM ABOVE
QTY: 1 LOC: OFFICE(GF)



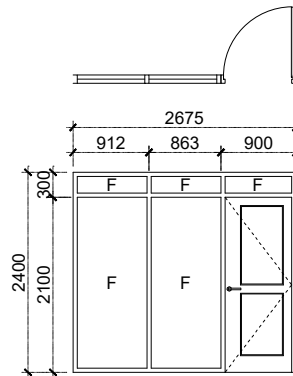
DW3
ALUMINUM FRAMED POWDER COATED SWING DOOR WITH 10mm THK. ONE-WAY GLASS AND FIXED GLASS WALL WITH TRANSOM ABOVE
QTY: 1 LOC: OFFICE(GF)



DW4
ALUMINUM FRAMED POWDER COATED SWING DOOR WITH 10mm THK. ONE-WAY GLASS AND FIXED GLASS WALL WITH TRANSOM ABOVE
QTY: 1 LOC: AUDITOR'S(GF)

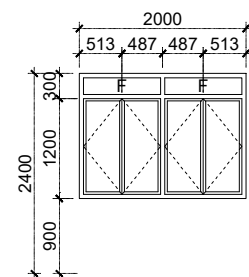


DW5
ALUMINUM FRAMED POWDER COATED SWING DOOR WITH 10mm THK. ONE-WAY GLASS AND FIXED GLASS WALL WITH TRANSOM ABOVE
QTY: 1 LOC: OFFICE(SF)



DW6
ALUMINUM FRAMED POWDER COATED SWING DOOR WITH 10mm THK. ONE-WAY GLASS AND FIXED GLASS WALL WITH TRANSOM ABOVE
QTY: 1 LOC: OFFICE(SF)

SCHEDULE OF COMBINED DOORS & WINDOWS
SCALE 1:100 MTS.



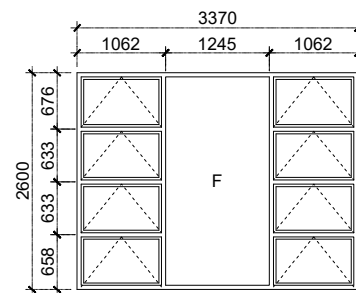
W1

ALUMINUM FRAMED POWDER
COATED CLOSET WINDOW WITH
10mm THK. ONE-WAY GLASS WITH
TRANSOM ABOVE

QTY: 19

LOC: CHIEF'S(GF)

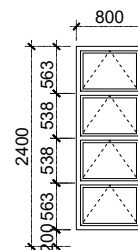
LOBBY(GF)
OFFICE(GF)
STORAGE(GF)
AUDITOR'S(GF)
CONFERENCE(SF)
EQUIPMENT(SF)
E.O.C.(SF)
STORAGE/STOCK(SF)



ALUMINUM FRAMED POWDER COATED
AWNING WINDOW AND FIXED WINDOW
WITH 10mm THK. ONE-WAY GLASS
WITH TRANSOM ABOVE

QTY: 1

LOC: LOBBY(GF)



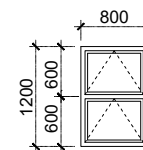
W3

ALUMINUM FRAMED POWDER
COATED AWNING WINDOW WITH
10mm THK. ONE-WAY GLASS
WITH TRANSOM ABOVE

QTY: 9

LOC: CHIEF'S(GF)

LOUNGE(GF)
DINING(GF)
CONFERENCE(SF)

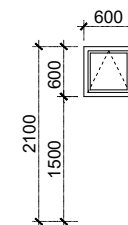


W4

ALUMINUM FRAMED POWDER
COATED AWNING WINDOW WITH
10mm THK. ONE-WAY GLASS
WITH TRANSOM ABOVE

QTY: 4

LOC: STAIR(GF&SF)

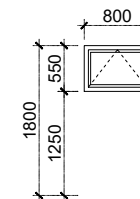



 ALUMINUM FRAMED POWDER
 COATED AWNING WINDOW WITH
 10mm THK. ONE-WAY GLASS
 WITH TRANSOM ABOVE

QTY: 6

LOC: MALE CR(GF&SF)

FEMALE CR(GF&SF)
CHIEF'S(GF)



W6

ALUMINUM FRAMED POWDER
COATED AWNING WINDOW WITH
10mm THK. ONE-WAY GLASS
WITH TRANSOM ABOVE

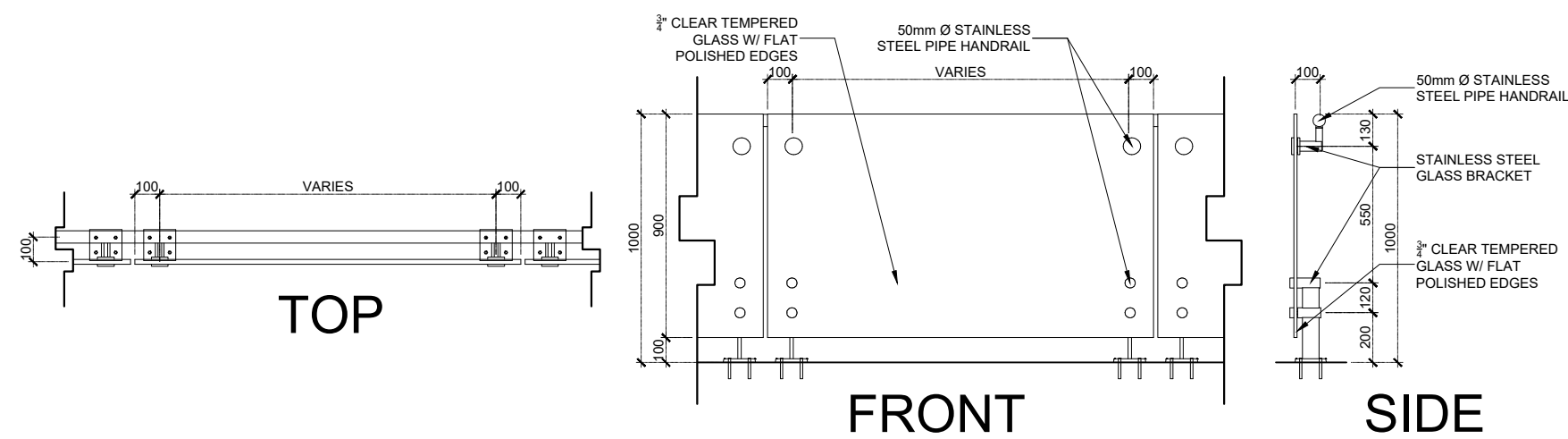
QTY: 2

LOC: KITCHEN(GF)

SCHEDULE OF WINDOWS

SCALE

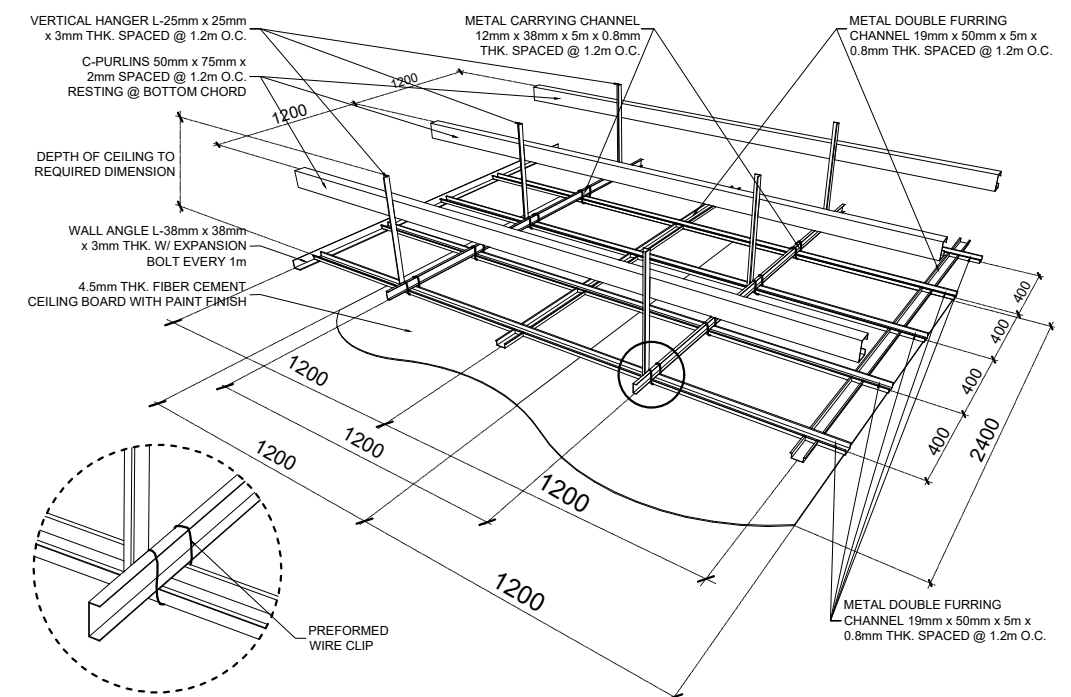
1:50 MTS.



RAILING DETAILS

SCALE

1:15 MTS.



CEILING DETAILS

SCALE

NTS.

GENERAL NOTES

SPECIFICATION

ALL CIVIL WORKS SHALL COMPLY WITH THE DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS (DPWH) STANDARD SPECIFICATIONS FOR PUBLIC WORKS STRUCTURES (BUILDINGS, PORTS, FLOOD CONTROL AND DRAINAGE STRUCTURES, AND WATER SUPPLY SYSTEMS) VOLUME III, REVISED 2019 ALONGSIDE WITH THE DESIGN GUIDELINES, CRITERIA AND STANDARDS (DGCS) VOLUME 3: WATER ENGINEERING PROJECTS 2015. THESE SPECIFICATIONS WILL LEAD TO AN EFFICIENT, ECONOMICAL AND IMPROVED QUALITY OF DESIGN FOR PUBLIC INFRASTRUCTURE, THUS GIVING SAFER AND MORE EFFICIENT PUBLIC INFRASTRUCTURE TO THE FILIPINO PEOPLE.

1.0 STANDARDS AND REFERENCES

- THE FOLLOWING SHALL GOVERN THE DESIGN FABRICATION AND CONSTRUCTION OF THE PROJECT.
- 1.1 NATIONAL STRUCTURAL CODE OF THE PHILIPPINES (N.S.C.P 2015) VOL. 1, SEVENTH EDITION.

2.0 DESIGN CRITERIA

2.1 LOADINGS

A. DEAD LOAD		B. LIVE LOAD	
CONCRETE	24.00 kN/m ³	ROOF	
STEEL	77.00 kN/m ³	- PURLINS	0.75 kPa
150 mm THK. CHB WALL	3.11 kPa	- TRUSS	0.75 kPa
100 mm THK. CHB WALL	2.98 kPa	OFFICES	1.90 kPa
PARTITION	1.00 kPa	TOILETS	
FLOOR FINISH	1.10 kPa	HALLWAY	4.80 kPa

BUILDING CATEGORY = 4 (STANDARD OCCUPANCY STRUCTURES)
EXPOSURE = B (URBAN AND SUBURBAN AREAS)
MAXIMUM WIND VELOCITY, V = 250 KPH

$$P = qh [(GCpf) - (GCpi)] \quad (\text{DESIGN WIND PRESSURE})$$

WHERE: qh = VELOCITY PRESSURE (kPa)

GCpf = EXTERNAL PRESSURE COEFFICIENT

GCpi = INTERNAL PRESSURE COEFFICIENT

EXPOSURE = B

D. SEISMIC LOAD

$$V = \frac{C_{vl} W}{RT} \quad (\text{DESIGN BASE SHEAR})$$

$$V_{max} = \frac{2.50 C_{vl} W}{RT} \quad V_{min} = 0.11 C_{vl} W$$
$$V_{min} = \frac{0.80 Z N_{vl}}{R} W \quad (\text{ZONE 4})$$

WHERE: W = TOTAL DEAD LOAD

T = NATURAL PERIOD = Ct (h)

WHERE: C = NUMERICAL COEFFICIENT

h = BUILDING HEIGHT

I = IMPORTANCE FACTOR = 1.00

R = NUMERICAL FACTOR = 8.50

SEISMIC COEFFICIENT Cv = 0.64 Nv

Ca = 0.44 NA

NEAR SOURCE FACTOR (9.4 km) Na = 1.00

Nv = 1.25

Z = SEISMIC ZONE = 0.40 (ZONE 4)

S = SOIL TYPE = D

DAMPING (FOR RESPONSE SPECTRUM) = 0.05

E. LOAD COMBINATION

ULTIMATE STRENGTH DESIGN

$$1.4 (D + F)$$

$$1.2 (D + F + T) + 1.6 (L + H) + 0.5 (Lr \text{ or } R)$$

$$1.2 (D) + 1.6 (Lr \text{ or } R) + (f1L \text{ or } 0.5W)$$

$$1.2 (D) + 1.0W + f1L + 0.5 (Lr \text{ or } R)$$

$$1.2 (D) + 1.0E + f1L$$

$$0.9(D) + 1.0W + 1.6H$$

$$0.9(D) + 1.0E + 1.6H$$

ALLOWABLE STRENGTH DESIGN (BASIC LOAD COMBINATION)

$$D + F$$

$$D + H + F + L$$

$$D + H + F + Lr$$

$$D + H + F + 0.75(L + Lr)$$

$$D + H + F + (0.6W \text{ or } E/1.4)$$

ALLOWABLE STRENGTH DESIGN

(ALTERNATE BASIC LOAD COMBINATION)

$$D + H + F + 0.75(L + Lr + (0.6W \text{ or } E/1.4))$$

$$0.6D + 0.6W + H$$

$$0.6D + E/1.4 + H$$

$$D + L + Lr$$

$$D + L + 0.6W$$

$$D + L + E/1.4$$

2.2 DESIGN STRESSES

A. CONCRETE COMPRESSIVE STRENGTH @ 28 DAYS:

a. FOOTINGS, COLUMNS, BEAMS AND SLABS

b. SLAB ON FILL

$$f_c = 20.7 \text{ MPa (3,000 psi)}$$

$$f_c = 20.7 \text{ MPa (3,000 psi)}$$

B. REINFORCING STEEL BARS

a. FOR BARS 16mm AND GREATER (INTERMEDIATE GRADE DEFORMED BAR)

b. FOR BARS LESS THAN 16mm (STRUCTURAL GRADE DEFORMED BAR)

$$f_y = 275 \text{ MPa (40,000 psi)}$$

$$f_y = 275 \text{ MPa (40,000 psi)}$$

C. STRUCTURAL STEEL ASTM-A36

FOR TRUSSES, BRACINGS, & STRUTS

$$f_y = 248 \text{ MPa (36,000 psi)}$$

D. PURLINS

COLD FORMED LIGHT

$$f_y = 248 \text{ MPa (36,000 psi)}$$

E. MASONRY UNIT (CHB)

NON - LOADING BEARING CHB WALLS

$$f_m' = 3.45 \text{ MPa (500 psi)}$$

$$E - 60XX ELECTRODE$$

$$a. F1 = 96.60 \text{ mPa (14, 000 psi)}$$

$$b. Fv = 69.00 \text{ mPa (10, 000 psi)}$$

3.0 IN THE INTERPRETATION OF THE DRAWING, INDICATED DIMENSIONS SHALL GOVERN.

DISTANCES AND SIZES SHALL NOT BE SCALED FOR CONSTRUCTIONS PURPOSES

4.0 IN REFERENCES TO OTHER DRAWINGS, SEE ARCHITECTURAL DRAWINGS FOR DEPRESSIONS IN FLOOR

SLABS, OPENINGS IN THE WALLS AND SLABS, INTERIOR PARTITIONS, LOCATIONS OF DRAINS ETC.

5.0 IN CASE OF DISCREPANCIES AS TO THE LAYOUT, DIMENSIONS AND ELEVATIONS BETWEEN THE

STRUCTURAL PLANS AND ARCHITECTURAL DRAWINGS, THE CONTRACTORS SHALL NOTIFY BOTH THE

STRUCTURAL ENGINEER AND ARCHITECTS.

6.0 ALL CONCRETE WORKS AND CONCRETE REINFORCEMENTS SHALL BE DONE IN ACCORDANCE WITH THE

ACI 318 95 BUILDING CODE REQUIREMENT AND ALL STRUCTURAL STEEL WORKS ACCORDING WITH THE

WITH THE AISC SPECIFICATION (9TH EDITION) IN SO FAR AS THEY DO NOT CONFLICT WITH THE LOCAL

BUILDING CODE REQUIREMENT.

7.0 ACI REFERS TO AMERICAN CONCRETE INSTITUTE, AISC REFERS TO AMERICAN INSTITUTE OF STEEL

CONSTRUCTION AND ASTM REFERS TO AMERICAN SOCIETY FOR TESTING MATERIALS.

8.0 CONSTRUCTION NOTES AND TYPICAL DETAILS APPLY TO ALL DRAWINGS UNLESS OTHERWISE SHOWN OR

NOTED. MODIFY TYPICAL DETAILS AS DIRECTED TO MEET SPECIAL CONDITIONS.

9.0 SHOP DRAWING WITH ERECTION AND PLACING DIAGRAMS OF ALL STRUCTURAL STEELS, MISCELLANEOUS

IRON, PRE-CAST CONCRETE, ETC. SHALL BE SUBMITTED FOR ENGINEERS APPROVAL BEFORE FABRICATION.

10. CONTRACTOR SHALL NOTE AND PROVIDE ALL MISCELLANEOUS CURBS, SILLS, STOOLS EQUIPMENT AND

MECHANICAL BASES THAT ARE REQUIRED BY THE ARCHITECTURAL, ELECTRICAL AND MECHANICAL DRAWINGS.

NOTES ON CONCRETE MIXES & PLACING

1. ALL CONCRETE SHALL DEVELOP A MIN. COMPRESSIVE STRENGTH AT THE END OF TWENTY EIGHT (28) DAYS W/ CORRESPONDING MAXIMUM SIZE AGGREGATE & SLUMP AS FOLLOWS.

LOCATION	28 DAYS STRENGTH	MAX. SIZE OF AGGREGATE	MAX SLUMP
ALL OTHERS, INCLUDING	3000 PSI (20.7 MPa)	20 mm	100mm
SUSPENDED SLABS			
COLUMNS	3000 PSI (20.7 MPa)	20 mm	100mm
BEAMS, SLABS	3000 PSI (20.7 MPa)	20 mm	100mm
SLAB ON FILL	2500 PSI (17.5 MPa)	20 mm	100mm

2. MAINTAIN MINIMUM CONCRETE COVER FOR REINFORCING STEEL AS FOLLOWS.

SUSPENDED SLABS	20mm
SLAB ON GRADE	40mm
WALLS ABOVE THE GRADE	25mm
BEAM STIRRUPS AND COLUMN TIES	40mm
WHERE CONCRETE IS EXPOSED TO	
EARTH BUT POURED AGAINST FORMS	50mm
WHERE CONCRETE IS DEPOSITED DIRECTLY AGAINST EARTH	75mm

3. CONCRETE SHALL BE DEPOSITED IN ITS FINAL POSITON WITHOUT SEGREGATION, RE-HANDLING OR PLACING SHALL BE DONE PREFERABLY WITH BUGGIES, BUCKETS OR WHEELBARROWS, NO CHUTES WILL BE ALLOWED EXCEPT TO TRANSFER CONCRETE FROM HOPPERS TO BUGGIES, WHEELBARROWS OR BUCKETS IN WHICH CASE THEY SHALL NOT EXCEED SIX (6) METERS IN AGGREGATE LENGTH.

4. NO DEPOSITING OF CONCRETE SHALL BE ALLOWED WITHOUT THE USE OF VIBRATORS UNLESS AUTHORIZED IN WRITING DESIGNER AND ONLY FOR UNUSUAL CONDITIONS WHERE VIBRATIONS ARE EXTREMELY DIFFICULT TO ACCOMPLISH.

5. ALL ANCHOR BOLTS, DOWELS, AND OTHER INSERTS SHALL BE PROPERLY POSITIONED & SECURED IN PLACE PRIOR TO PLACING OF CONCRETE.

6. ALL CONCRETE SHALL BE KEPT MOST FOR A MINIMUM OF SEVEN CONSECUTIVE DAYS IMMEDIATELY AFTER POURING BY THE USE O WET BURLAP, FOG SPRAYING, CURING COMPOUNDS OR OTHER APPROVED METHODS.

7. STRIPPING OF FORMS AND SHORES:
- | | |
|------------------------------|----------|
| FOUNDATION | 24 HOURS |
| SUSPENDED SLAB EXCEPT WHEN | 8 DAYS |
| ADDITIONAL LOADS ARE IMPOSED | |
| WALLS | 21 DAYS |
| BEAMS | 14 DAYS |
| COLUMNS | 21 DAYS |

8. THE CONTRACTOR SHALL SUBMIT THE SCHEDULE OF POURING AND THE LOCATION OF THE CONSTRUCTION JOINTS TO THE STRUCTURAL ENGINEER AT LEAST (4) DAYS PRIOR TO THE POURING FOR APPROVAL.

9. THE CONTRACTOR SHALL FURNISH AND MAINTAIN ADEQUATE FORMS AND SHORINGS UNTIL THE CONCRETE MEMBERS HAVE ATTAINED THEIR WORKING CONDITION AND STRENGTH.

NOTES ON FOOTINGS

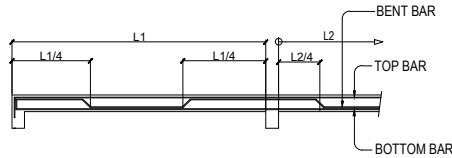
1. FOOTINGS ARE DESIGNED FOR AN ALLOWANCE SOIL BEARING PRESSURE OF 65 kPa (1360psf). CONTRACTOR SHALL REPORT TO THE ENGINEER, IN WRITING, THE ACTUAL SOIL CONDITIONS UNCOVERED AND CONFIRM ACTUAL BEARING CAPACITY OF SOIL BEFORE DEPOSITING CONCRETE.
2. FOOTING SHALL REST AT LEAST 150mm BELOW NATURAL GRADE LINE UNLESS OTHERWISE INDICATED IN PLANS. NO FOOTING SHALL REST ON FILL.
3. MINIMUM CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE 75mm CLEAR FOR CONCRETE DEPOSITED THE GROUND AND 50mm FOR CONCRETE DEPOSITED AGAINST A FORMWORK.
4. IN CASES WHERE THE SOIL CONDITION IS SUCH THAT THE MINIMUM ALLOWABLE SOIL PRESSURE OF 96kPa (2000 psf) CAN NOT BE ATTAINED AT A PRACTICAL DEPTHS THE USE OF MICROPILES, BORED PILES, OR DRIVEN PILES MAY BE ADOPTED IN LIEU OF STANDARD ISOLATED FOOTINGS.

NOTES ON REINFORCEMENT

1. UNLESS OTHERWISE NOTED IN PLANS, THE YIELD STRENGTH OF REINFORCING BARS SHALL BE:
- | | |
|---|--------------------------------------|
| A. FOOTINGS, FOOTING BEAMS AND GIRDERS | $f_y = 275 \text{ MPa (40,000 psi)}$ |
| B. COLUMNS AND SHEAR WALLS | $f_y = 275 \text{ MPa (40,000 psi)}$ |
| C. BEAMS AND GIRDER | $f_y = 275 \text{ MPa (40,000 psi)}$ |
| D. NON-LOAD BEARING WALL PARTITIONS, BEDDED SLABS, FLOOR & ROOF SLABS, PARAPETS, CATCH BASIN, SIDE WALK | $f_y = 275 \text{ MPa (40,000 psi)}$ |
2. ALL REINFORCING BARS SIZE 10mm OR LARGER SHALL BE DEFORMED IN ACCORDANCE WITH THE ASTM A-706 BARS SMALLER THAN 10mm MAY BE PLAIN.
3. SPLICES SHALL BE SECURELY WIRED TOGETHER & SHALL LAP OR EXTEND IN ACCORDANCE w/ TABLE B (TABLE OF LAP SPLICE & ANCHORAGE LENGTH) UNLESS OTHERWISE SHOWNON DRAWINGS, SPLICES SHALL BE STAGGERED WHENEVER POSSIBLE.

NOTES ON CONCRETE SLABS

1. ALL SLAB REINFORCEMENTS SHALL BE 20mm CLEAR MINIMUM FROM BOTTOM AND FROM THE TOP OF SLAB.
2. UNLESS OTHERWISE SHOWN, REINFORCEMENT IN CONTINUOUS ELEVETED SLAB SHALL BE CUT AS FOLLOWS:



3. IF SLABS AR E REINFORCED BOTHWAYS BARS ALONG THE SHORTER SPAN SHALL BE PLACED BELOW THOSE ALONE THE LONG SPAN AT THE CENTER AND OVER THE LONGER SPAN FOR REINFORCING BARS NEAR THE SUPPORTS. THE SPACING OF THE BARS AT THE COLUMN STRIPS SHALL NOT BE MORE THAN ONE AND A HALF (1 1/2) SLAB THICKNESS.

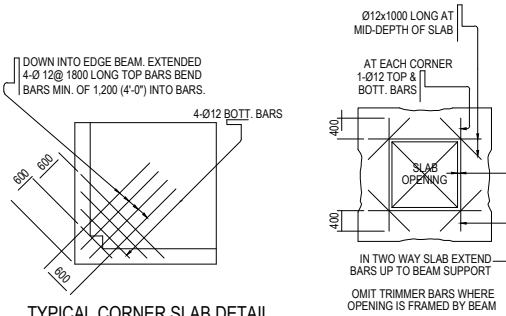
4. TEMPERATURE BARS FOR SLAB SHALL BE GENERALLY PLACED NEAR THE FACE IN TENSION AND SHALL NOT BE LESS THAN 0.0025 X GROSS-SECTIONAL AREA (Ag) OF THE SLAB. (SEE SCHEDULE BELOW)

SCHEDULE OF MINIMUM SLAB REINFORCEMENT	
MINIMUM TEMPERATURE BARS	
100 mm	10mm Ø @ 250mm EACH WAY
125 mm	10mm Ø @ 250mm EACH WAY
150 mm	10mm Ø @ 250mm EACH WAY
175 mm	10mm Ø @ 250mm EACH WAY
200 mm	10mm Ø @ 250mm EACH WAY

5. UNLESS OTHERWISE NOTED IN THE PLANS ALL BEDDED SLABS SHALL BE REINFORCED WITH 10mm Ø AT 250mm O.C. EACH WAY TO CENTER OF SLAB AND CONSTRUCTION JOINTS FOR SAME SHALL NOT BE LESS THAN 3.65 METER APART.

6. PROVIDE EXTRA REINFORCEMENTS FOR CORNER SLAB (TWO ADJACENT DISCONTINUOUS EDGES) AS SHOWN BELOW.

7. CONCRETE SLAB REINFORCEMENT BE PROPERLY SUPPORTED WITH 10mm STEEL CHAIR OR APPROVED EQUIVALENT SPACED AT 1.0 METER ON CENTER BOTHWAYS.

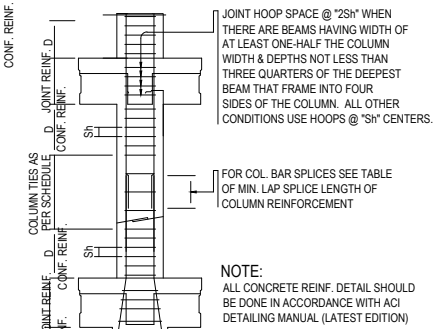
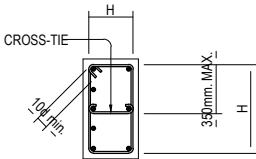


TYPICAL CORNER SLAB DETAIL

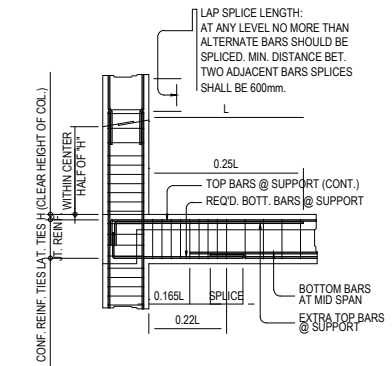
TYPICAL SLAB OPENING DET.

NOTES ON COLUMNS

1. PROVIDE EXTRA SETS OF TIES AT 100 O.C. FOR TIED COLUMN REINFORCEMENT ABOVE AND BELOW BEAM-COLUMN CONNECTIONS FOR A DISTANCE FROM FACE OF CONNECTION EQUAL TO GREATER OF THE OVERALL THICKNESS OF COLUMN, 1/6 THE CLEAR HEIGHT OF COLUMN OR 450mm.
2. COLUMN TIES SHALL BE PROTECTED EVERYWHERE BY A COVERING OF CONCRETE CAST MONOLITHICALLY WIT HTHE CORE WITH A MINIMUM THICKNESS OF 40mm AND NOT LESS THAN 40 TIMES THE MAXIMUM SIZE OF COARSE AGGREGATE IN MILLIMETERS.
3. WHERE COLUMNS CHANGE IN SIZE, VERTICAL REINFORCEMENT SHALL BE OFFSET AT A SLOPE MONOLITHICALLY WITH THE CORE WITH MINIMUM THICKNESS OF 40mm AND NOT LESS THAN 40 TIMES THE MAXIMUM SIZE COARSE AGGREGATE IN MILLIMETERS
4. UNLESS OTHERWISE INDICATED IN THE PLANS, LAP SPLICES FOR VERTICAL COLUMN REINFORCEMENT SHALL BE MADE WITHIN THE CENTER HALF OF COLUMN HEIGHT, AND THE SPLICE LENGTH SHALL BE LESS THAN 40 BAR DIAMETERS. WELDING OR APPROVED MECHANICAL DEVICES MAY BE USED PROVIDED THAT NOT MORE THAN ALTERNATE BARS ARE WELDED OR MECHANICALLY SPLICED AT ANY LEVEL AND THE VERTICAL DISTANCES BETWEEN THESE WELDS OR SPLICES OF ADJACENT BARS IS NOT LESS THAN 600mm.



TYPICAL COLUMN ELEV. SHOWING DOWELS AND TIES SPACING



TYP. DETAIL OF COL. LAP SPLICE & EXT. GIRDER TO COL. CONNECT.

NOTES ON BEAMS AND GIRDERS

1. UNLESS, OTHERWISE NOTED IN PLANS, CAMBER ALL BEAMS AND GIRDER AT LEAST 6mmØ FOR EVERY 4.50 M OF SPAN, EXCEPT CANTILEVERS FOR WHICH THE CAMBER SHALL BE AS NOTED IN PLANS OR AS ORDERED BY THE ENGINEER BUT IN NO CASE LESS THAN 20 mm FOR EVERY 3.0 M OF FREE SPAN.
2. TYPICAL BARS BENDING AND CUTTING DETAILS FOR BEAMS SHALL BE AS SHOWN IN FIG. B-1

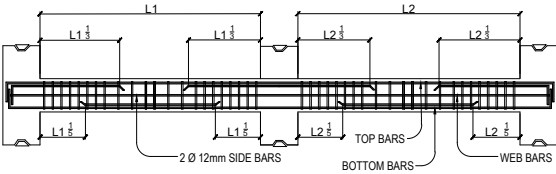


FIGURE B-1

GENERAL CONSTRUCTION NOTES

TABLE 'A' TENSION BARS TABLE OF LAP SPICE & ANCHORAGE LENGTH (mm)				
BAR SIZES (DEFORMED)	f _c = 20.7 MPa (3000 psi)		f _c = 27.6 MPa (4000 psi)	
	EMBEDMENT	LAPPED	EMBEDMENT	LAPPED
Ø10	300	300	300	300
Ø12	300	300	300	300
Ø16	300	400	300	400
Ø20	400	500	350	500
Ø25	600	800	550	750
Ø28	750	1000	650	850
Ø32	950	1300	850	1100

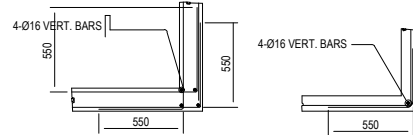
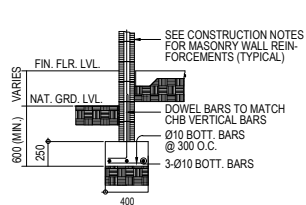
NOTES:
1. TOP PLAIN BARS, MULTIPLY VALUE BY 2
2. NOT MORE THAN 33% OF THE BARS SHALL BE SPLICED WITHIN THE REQUIRED LAP LENGTH
3. VALUES GIVEN ABOVE CAN ALSO BE USED FOR COLUMNS

TABLE 'B' COMPRESSION BARS TABLE OF LAP SPICE & ANCHORAGE LENGTH (mm)				
BAR SIZES (DEFORMED)	f _c = 20.7 MPa (3000 psi)		f _c = 27.6 MPa (4000 psi)	
	EMBEDMENT	LAPPED	EMBEDMENT	LAPPED
Ø10	225	300	200	300
Ø12	275	300	250	300
Ø16	350	400	325	400
Ø20	450	500	475	500
Ø25	550	625	550	625
Ø28	625	675	625	675
Ø32	700	775	700	775

NOTES:
1. TOP PLAIN BARS, MULTIPLY VALUE BY 2
2. NOT MORE THAN 33% OF THE BARS SHALL BE SPLICED WITHIN THE REQUIRED LAP LENGTH
3. VALUES GIVEN ABOVE CAN ALSO BE USED FOR COLUMNS

REINFORCING CONCRETE LINTEL BEAMS IN CONCRETE BLOCK WALLS

LINTELS IN BLOCK WALLS						
CLEAR SPAN (L)	TOTAL LENGTH (L+0.40M)	MIN. f _c (MPa)	HEIGHT OF LINTEL (mm)	REINFORCEMENT		
				BOTTOM	TOP	STIRRUPS
1.20 M	1.60 M	14.0	200	1-Ø10	1-Ø10	Ø6 mm @ 200mm
1.50 M	1.90 M		200	1-Ø10	1-Ø10	Ø6 mm @ 200mm
1.80 M	2.20 M		200	1-Ø12	1-Ø10	Ø6 mm @ 200mm
2.10 M	2.50 M	17.0	250	1-Ø12	1-Ø10	Ø6 mm @ 200mm
2.40 M	2.80 M		250	1-Ø12	1-Ø10	Ø6 mm @ 200mm
2.70 M	3.10 M		250	1-Ø16	1-Ø12	Ø10mm @ 200mm
3.00 M	3.40 M	20.0	300	1-Ø16	1-Ø12	Ø10mm @ 200mm
3.30 M	3.70 M		300	1-Ø16	1-Ø12	Ø10mm @ 200mm
3.60 M	4.00 M		300	1-Ø20	1-Ø12	Ø10mm @ 200mm



TYPICAL CONNECTION DETAIL OF R.C. WALL AT CORNERS

NOTES ON WELDS

- USE E60xx ELECTRODES FOR ALL MEMBERS WELDED.
- WELDS SHALL DEVELOP THE FULL STRENGTH OF MEMBERS JOINED UNLESS OTHERWISE SHOWN OR DETAILED IN THE DRAWINGS.

NOTES ON STRUCTURAL STEEL

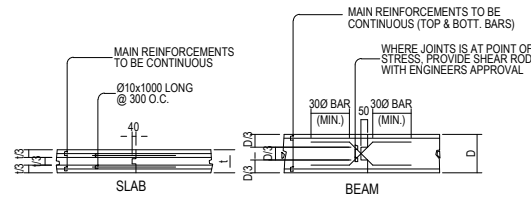
- STRUCTURAL STEEL TO BE USED FOR FABRICATION AND ERECTION OF THIS STRUCTURE SHALL COMPLY WITH ALL THE PERTINENT PROVISION OF AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDING LATEST EDITION.
- ALL STRUCTURAL STEEL SHAPES SHALL CONFORM TO ASTM A36 STRUCTURAL STEEL UNLESS OTHERWISE INDICATED.
- ALL WELDED CONNECTIONS SHALL DEVELOP THE FULL STRENGTH OF THE MEMBERS CONNECTED.
- UNLESS OTHERWISE SPECIFIED ALL WELDING RODS SHALL CONFORM WITH E60 ELECTRODES.
- ALL BOLTS USED UNLESS OTHERWISE SPECIFIED SHALL BE ASTM A307 BOLTS.

NOTES ON EMBED PIPES

- ALL EMBEDDED PIPES FOR UTILITIES ETC. THAT PASS THRU BEAMS SHALL NOT EXCEED 100mm IN DIAMETER OR 1/3 BEAM DEPTH WHICHEVER IS LESS, UNLESS OTHERWISE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.
- NO PIPES SHALL BE ALLOWED TO PASS THRU BEAMS VERTICALLY.
- NO PIPES SHALL BE EMBEDDED IN COLUMNS.

NOTES ON CONSTRUCTION JOINTS IN CONCRETE

- WHERE A CONSTRUCTION JOINT IS TO BE MADE, THE SURFACE OF CONCRETE SHALL BE CLEANED AND ALL LAITANCE AND STANDING WATER REMOVED SHEAR KEY SHALL BE PROVIDED AT THE JOINT.



TYPICAL SLAB & BEAM CONSTRUCTION JOINT DET.

NOTES ON CONCRETE WALLS

- ALL WALLS SHALL BE REINFORCED ACCORDING TO THE FOLLOWING SCHEDULE OF WALL REINFORCEMENT UNLESS OTHERWISE INDICATED IN THE PLANS.

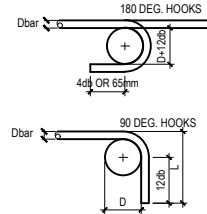
WALL THICKNESS	REINFORCEMENT		REMARKS	VERTICAL SECTION
	HORIZONTAL	VERTICAL		
100mm	Ø10mm @ 250mm O.C.	Ø10mm @ 300mm O.C.	HORIZONTAL BARS AT CENTERS VERTICAL BARS STAGGED OUT	VER. BARS HOR. BARS
125mm	Ø10mm @ 200mm O.C.	Ø10mm @ 250mm O.C.		
150mm	Ø12mm @ 250mm O.C.	Ø12mm @ 300mm O.C.		

REINFORCING BARS SHALL HAVE 25mm CLEAR CONCRETE COVER FROM FACE OF WALL EXCEPT FOR WALLS IN CONTACT WITH THE GROUND WHERE A MINIMUM OF 60mm SHALL BE PROVIDED AND FOR EXPOSED FACES OF FORMED WALLS WHERE THE MINIMUM SHALL BE 50mm CLEAR.

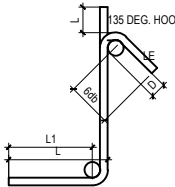
- CARRY VERTICAL BARS AT LEAST 60mm ABOVE FLOOR LEVEL TO PROVIDE FOR SPLICES WHEN NECESSARY STOP AT 50mm BELOW TOP SLAB OR SOLID BAND WHERE THE WALL ENDS VERTICAL AND HORIZONTAL BARS SHALL BE SPLICED BY LAPPING A DISTANCE EQUAL TO 30 DIAMETERS AND WIRED SECURELY WITH 16 G.I. WIRE PROVIDED THAT SPLICES IN ADJACENT BARS ARE STAGGERED AT LEAST 1.50M O.C.
- UNLESS OTHERWISE NOTED IN THE PLANS, ALL OPENINGS IN WALLS 250mm OR THICKER SHALL BE REINFORCED AROUND WITH 2-20mmØ BARS. FOR 225mm, 200mm, 175mm, 150mm THICK WALLS, USE 2-16mmØ. FOR 125mm AND 100mm THICK WALLS, USE 2-12mmØ BARS. ALL WALLS SPANNING SHALL HAVE VERTICAL REINFORCEMENT BENT A U-FORM LIKE STIRRUPS AND SPACED ACCORDING TO THE SCHEDULE UNLESS OTHERWISE NOTED.

NOTES ON STIRRUPS

- ALL REINFORCEMENT SHALL BE BENT COLD UNLESS OTHERWISE PERMITTED BY THE STRUCTURAL ENGINEER.
- AS SHOWN IN THE DESIGN DRAWINGS OR PERMITTED BY THE STRUCTURAL ENGINEER.
- TIES & CLOSE STIRRUPS MUST BE AT 135.



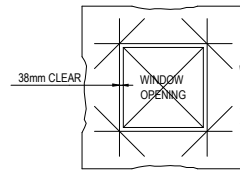
MAIN BAR END HOOKS (ALL GRADES)				
BAR SIZE (DEFORMED)	DIAMETER (mm)	180° HOOK	90° HOOK	
		D+2db	L	L
10 mm Ø	60	75	125	150
12 mm Ø	75	100	150	200
16 mm Ø	95	125	175	250
20 mm Ø	115	150	200	300
25 mm Ø	150	200	230	450
28 mm Ø	240	300	350	550
32 mm Ø	300	335	450	600



STIRRUP AND THE TIE HOOKS (ALL GRADES)				
BAR SIZE (DEFORMED)	DIAMETER (mm)	180° HOOK	90° HOOK	
		D+2db	L	L
10 mm Ø	40	125	85	100
12 mm Ø	50	165	115	115
16 mm Ø	65	200	140	150
20 mm Ø	115	300	165	300
32 mm Ø	150	335	230	405

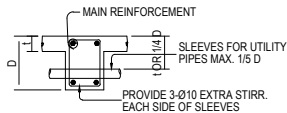
NOTE:
PROVIDE THESE ADDITIONAL BARS FOR ALL OPENINGS PLUS BARS (NOT SHOWN) PARALLEL TO SIDE OF OPENING EQUAL TO THE NUMBER OF TERMINATED BARS AT OPENING

SEE ARCHITECTURAL & MECHANICAL PLANS FOR SLAB OPENING LOCATION.

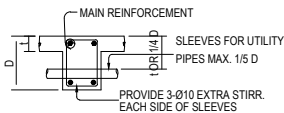


TYP. EXTERIOR WDW. & DOOR OPENING

- IF THE BEAM REINFORCING BARS END IN A WALL, THE CLEAR DISTANCE FROM THE BAR TO THE FARTHER FACE OF THE WALL IS NOT LESS THAN 25mm. EMBEDMENT LENGTH SHALL BE SHOWN IN A TABLE 'A' FOR TENSION BARS AND TABLE 'B' FOR COMPRESSION BARS UNLESS OTHERWISE SPECIFIED IN PLAN. TOP BARS AND SHALL NOT BE SPLICED WITHIN THE COLUMN OR TWO STIRRUPS SHALL BE PROVIDED AT ALL SPLICES.
- IF THERE ARE TWO OR MORE LAYERS OF REINFORCING BARS, USED 25mm BAR SEPARATORS SPACED AT 1.0M ON CENTER ON NO CASE SHALL THERE BE THAN TWO (2) SEPARATORS BETWEEN LAYERS OF BARS
- MINIMUM CONCRETE PROTECTION FOR REINFORCING BARS OR STEEL SHAPES SHALL BE AS SHOWN IN FIGURE B-2 UNLESS ELSEWHERE.



TYP. DET. FOR SLEEVES THRU CONCRETE BEAM
FIG. B-2



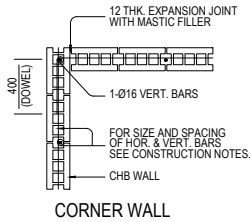
TYP. DET. FOR SLEEVES THRU CONCRETE BEAM
FIG. B-3

- WHEN A BEAM CROSSES A GIRDER, REST BEAM ON TOP OF GIRDER BARS, BEAM REINFORCING BARS SHALL BE SYMMETRICAL ABOUT THE CENTER LINE WHENEVER POSSIBLE.
- GENERALLY, NO SPLICES SHALL BE PERMITTED AT POINTS WHERE CRITICAL BENDING STRESSES OCCUR, SPLICES WHERE SO PERMITTED SHALL BE INDICATED IN TABLE 'A' AND 'B'. WELDED SPLICES SHALL DEVELOP IN TENSION AT LEAST 125% OF THE SPLICED YIELD STRENGTH OF THE BAR NOT MORE THAN 50% OF THE BARS AT ANY ONE SECTION IS ALLOWED TO BE SPLICED THEREIN.

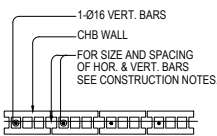
NOTES ON CONCRETE HOLLOW BLOCKS WALLS

- UNLESS OTHERWISE SHOWN IN PLANS ALL CONCRETE HOLLOW BLOCKS AND CERAMIC BLOCKS SHALL BE REINFORCED AS SHOWN IN THE SCHEDULE OF CONCRETE HOLLOW BLOCKS AND CERAMIC BLOCK REINFORCEMENT.
- PROVIDE 150mm x 300mm STIFFENER COLUMN REINFORCED WITH 4-12mm WITH 10mm Ø TIES AT 150mm ON CENTER WHERE CONCRETE HOLLOW BLOCK TERMINATES AND AT EVERY 3.0M LENGTH OF CONCRETE HOLLOW BLOCK WALLS UNLESS NOTED IN STRUCTURAL PLANS.

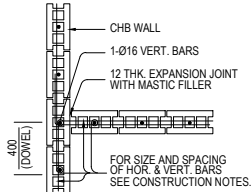
NOTES ON CONCRETE HOLLOW BLOCKS WALLS REINFORCEMENTS			
BLOCK THICKNESS	REINFORCEMENT		NOTES
	HORIZONTAL	LAPPED	
75 mm	10mm Ø EVERY 3RD LEVEL	10mm Ø @ 600mm O.C.	A. MINIMUM LAPS AT SPLICE= 0.25 M
125 mm	10mm Ø EVERY 3RD LEVEL	10mm Ø @ 600mm O.C.	B. PROVIDE RIGHT ANGLED REINFORCEMENT AT CORNERS 0.92 m LONG
150mm	10mm Ø EVERY 3RD LEVEL	10mm Ø @ 600mm O.C.	C. WHERE CHB OR CER. BLK. WALL DOWELS WITH THE SAME SIZE AS VER. OR HOR. REINFORCEMENT SHALL BE PROVIDED
200 mm	12mm Ø EVERY 3RD LEVEL	10mm Ø @ 600mm O.C.	



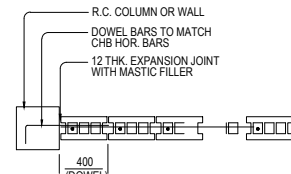
CORNER WALL



OPENING OR END WALL

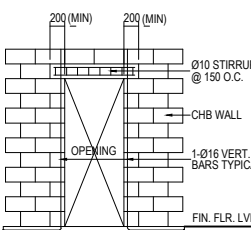


INTERSECTION WALL

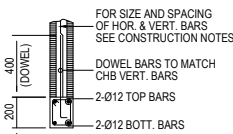


INTERSECTING R.C. COLUMN OR WALL

TYPICAL CONNECTION DETAIL OF MASONRY WALL

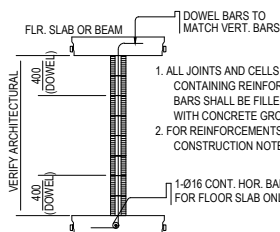


ELEVATION



SECTION

TYP. DET. OF LINTEL BEAM AT CHB WALL OPENING



TYP. SECTION OF MASONRY PARTITION REINFORCEMENTS



REPUBLIC OF THE PHILIPPINES
DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS
REGION IV - B
MINDORO ORIENTAL DISTRICT ENGINEERING OFFICE

BRGY. MASIPIT

CALAPAN CITY

PROJECT :

REHABILITATION OF MAINTENANCE
SECTION AND COA BUILDING

LOCATION: CALAPAN CITY, ORIENTAL MINDORO

SHEET CONTENT:

- GENERAL CONSTRUCTION NOTES

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RECOMMENDING APPROVAL:

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ASST. DISTRICT ENGINEER

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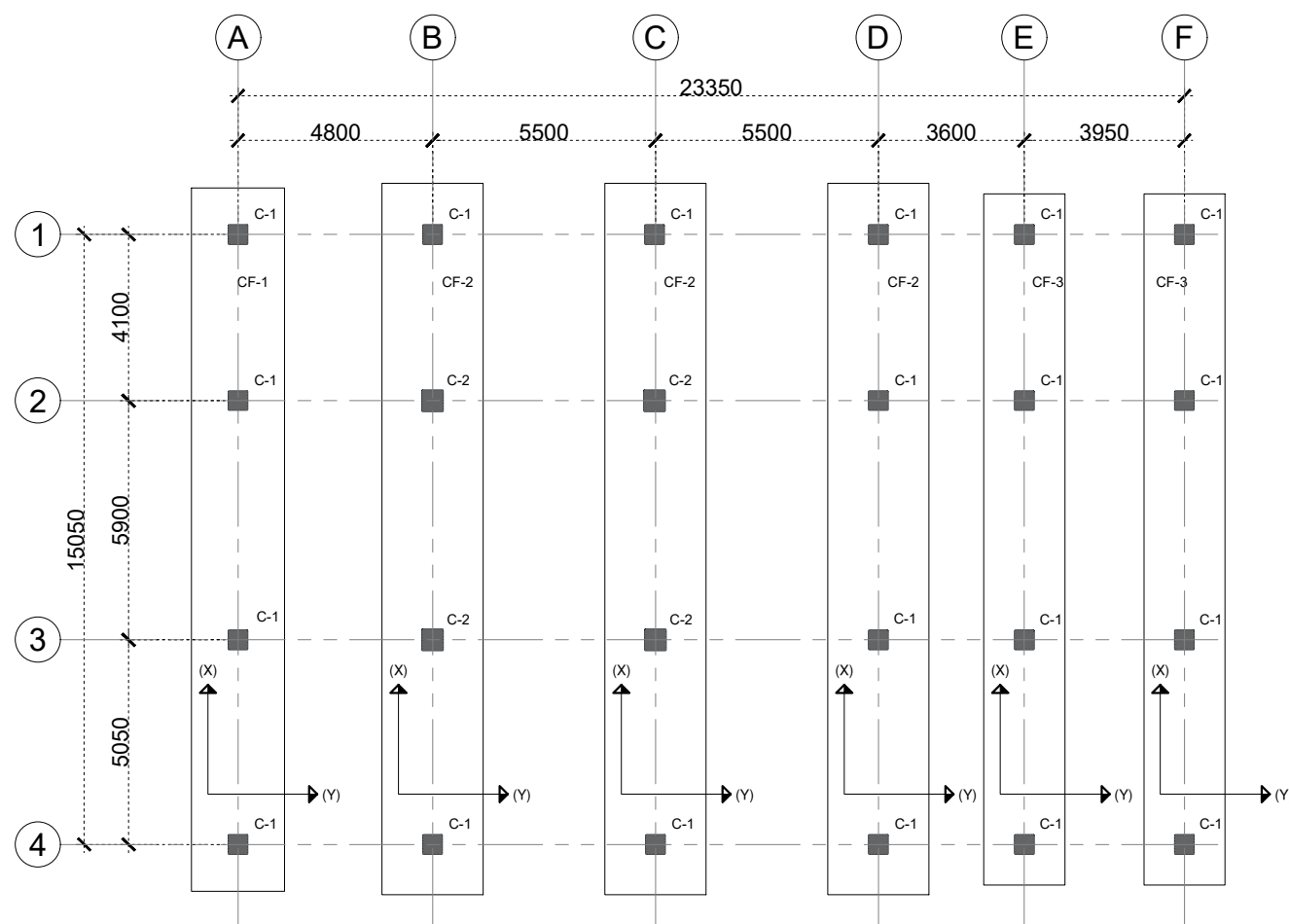
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MODEO

SHEET NO.

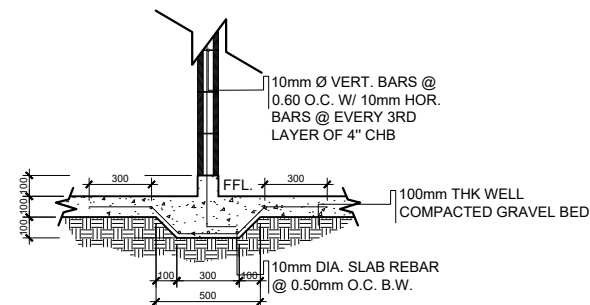
BUILDING
S 2



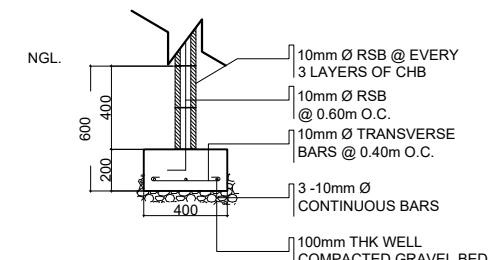
FOUNDATION PLAN
SCALE 1:100 MTS.

SLAB MARKED	FOOTING THICKNESS (mm)	LONGITUDINAL REINFORCEMENT		TRANSVERSE REINFORCEMENT	
		TOP (X)	BOTTOM (X)	TOP (Y)	BOTTOM (Y)
CF-1	2300 X 17350 X 450	20mm SPACED @ 120mm O.C.	16mm SPACED @ 120mm O.C.	20mm SPACED @ 295mm O.C.	16mm SPACED @ 295mm O.C.
CF-2	2500 X 17550 X 450	20mm SPACED @ 110mm O.C.	16mm SPACED @ 110mm O.C.	20mm SPACED @ 295mm O.C.	16mm SPACED @ 295mm O.C.
CF-3	2000 X 17050 X 450	20mm SPACED @ 110mm O.C.	16mm SPACED @ 125mm O.C.	20mm SPACED @ 295mm O.C.	16mm SPACED @ 295mm O.C.

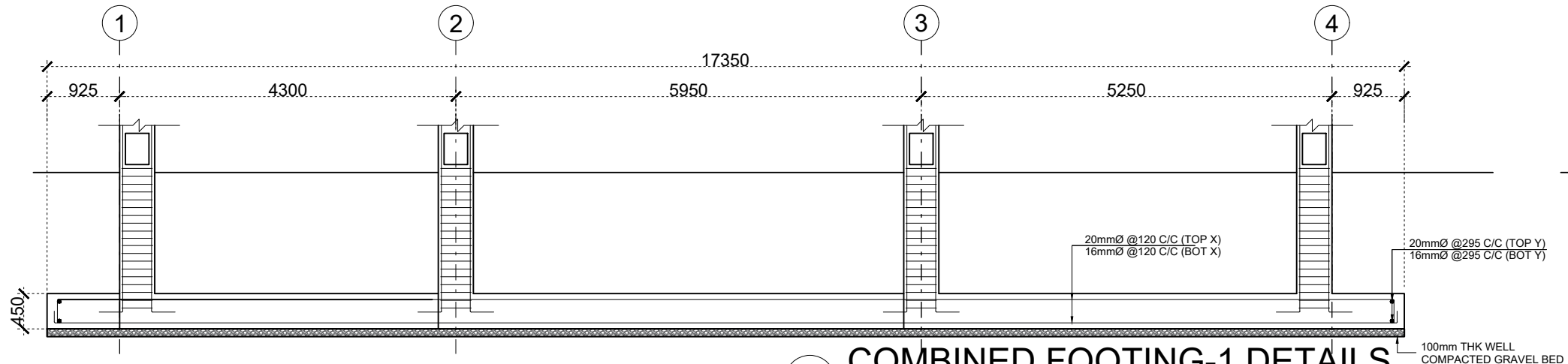
FOUNDATION SCHEDULE
SCALE NTS.



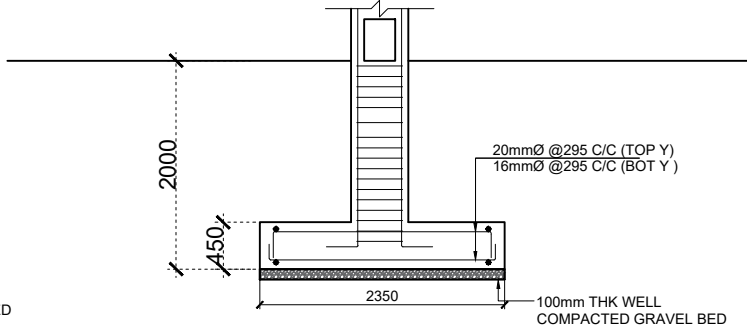
WF-1 DETAILS
SCALE 1:20 MTS.



WF-2 DETAILS
SCALE 1:20 MTS.



COMBINED FOOTING-1 DETAILS
SCALE 1:40 MTS.



 REPUBLIC OF THE PHILIPPINES DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS REGION IV - B MINDORO ORIENTAL DISTRICT ENGINEERING OFFICE BRGY. MASIPIT CALAPAN CITY	PROJECT :	SHEET CONTENT:	DRAFTED BY:	REVIEWED BY:	SUBMITTED BY:	RECOMMENDING APPROVAL:	APPROVED BY:	SET NO.	SHEET NO.
	REHABILITATION OF MAINTENANCE SECTION AND COA BUILDING	• FOUNDATION PLAN • FOUNDATION SCHEDULE • COMBINED FOOTING-1 DETAILS • WF-1 DETAILS • WF-2 DETAILS	AARON LOUIE C. ALULOD ENGINEER II	DUANE ANDREW ORIEL C. ANORICO ENGINEER II	EVELYN E. PENA ENGINEER III	ALBIEN M. MAHIA ASST.DISTRICT ENGINEER	SALVADOR E. ABAD DISTRICT ENGINEER	DPWH CY-2025 MODEO	BUILDING S 3
	LOCATION: CALAPAN CITY, ORIENTAL MINDORO		PREPARED BY: RAMIR G. BASA ENGINEER II						